

MEMORANDUM

Date: December 11, 2013

To: Brian Silveira

From: Tamar Fuhrer, AICP and Anjum Bawa, AICP

Subject: Transportation Assessment of 1414 Main Street

LA13-2628.00

This memorandum presents a transportation assessment Fehr & Peers conducted for the proposed mixed-use residential/commercial project located at 1414 Main Street in the Venice community of Los Angeles, California. This memorandum is presented in seven sections. First, the project and land use is described. Next, the trip generation forecasts are presented, followed by a discussion on site access and trip distribution. The traffic assessment and impact analysis is then presented. After this, cut-through traffic on Toledo Court is discussed. Finally, the results of this analysis are summarized.

PROJECT DESCRIPTION

The proposed project is located at 1414 Main Street in the Venice community of Los Angeles, California. The project is situated at the corner of Horizon Avenue & Main Street. The project would replace two existing apartment buildings, totaling 10 units, and two existing single-family homes. The new project would be a combination of residential and retail land use as follows:

- 26 condominium units
- 1,184 square feet of retail use
- 4,567 square feet of restaurant use

Additionally, there are 861 square feet of space that will be held for community use. Activities taking place in this space will be held outside of peak hours. The parking proposed for this project will be composed of an automated parking facility with 242 spaces, which will be accessible from the adjacent alley. Figure 1 shows a conceptual site plan.

TRIP GENERATION

Trip generation rates adopted in the Coastal Transportation Corridor Specific Plan (City of Los Angeles) and rates recommended in *Trip Generation*, 9th Edition (Institute of Transportation Engineers [ITE], 2012), were used to estimate trips for the proposed residential component. Pass-by trip credit was applied to the commercial components of the project, per *Traffic Impact Study Guidelines* (City of Los Angeles, May 2012). Internal trip capture credit was also applied to the commercial uses to account for trips that are expected to occur within proposed new components, without external vehicle trips being generated.



As shown in Table 1, the project is expected to generate 421 net daily trips, including nine trips during the AM peak hour, and 40 trips during the PM peak hour.

SITE ACCESS

The project's proposed vehicular access is consistent with Section 10.F.5(a) of the Venice Coastal Specific Plan (adopted December 2, 2003), which states:

Driveways and vehicular access to the Venice Coastal Development Projects shall be provided from alleys, unless the Department of Transportation determines that it is not Feasible.

Vehicular access to the project will be from the alley adjacent to the project site, which runs parallel to Main Street. Motorists will be able to enter the alley via Horizon Avenue or Market Street depending on their direction of approach. Horizon Avenue is a one-way street with westbound access only, so motorists traveling on Main Street will likely use Market Street to access the project parking. During egress, motorists will be able to use both Horizon Avenue and Market Street to turn onto Main Street.

TRIP DISTRIBUTION ON ADJACENT STREETS AND INTERSECTIONS

Using the aforementioned information regarding available access from adjacent streets, trips associated with the project were distributed through the adjacent street network using the following sub-regional distribution:

- North 30%
- South 20%
- West 10%
- East 40%

Figure 2 shows distribution of project trips on adjacent streets. As shown, a maximum of 18 trips are expected to be added during the PM peak hour to the intersection of Main Street & Horizon Street. During the same peak hour, a total of 29 trips are expected to be added to the intersection of Main Street & Market Street.

TRAFFIC ASSESSMENT

Existing Conditions

In conjunction with LADOT staff, the following two signalized intersections were selected for the study:

- 1. Main Street & Horizon Avenue
- 2. Main Street & Abbot Kinney Boulevard

Weekday AM and PM peak hour turning movement counts were collected at two study intersections in November 2013. Traffic count sheets are contained in Attachment A.

A variety of standard methodologies are available to analyze LOS. According to *Traffic Study Policies and Procedures*, this study is required to use the Critical Movement Analysis (CMA) method of intersection capacity calculation (Transportation Research Board, 1980) to analyze signalized intersections. The CMA methodology determines the intersection V/C ratio. The V/C ratio is then used to find the corresponding



LOS based on the definitions in Table 2. Under the CMA methodology, a V/C ratio is generated for each study intersection based on factors such as the volume of traffic and the number of lanes providing for such vehicle movement and an LOS grade. LOS worksheets are included in Attachment B.

The City of Los Angeles' Automated Traffic Surveillance and Control (ATSAC) system is a computer-based traffic signal control system that monitors traffic conditions and system performance to allow ATSAC-operations to manage signal timing to improve traffic flow conditions. The Adaptive Traffic Control System (ATCS) is an enhancement to ATSAC and provides fully traffic-adaptive signal control based on real-time traffic conditions. Both of the study intersections are assumed to be operating under ATSAC under existing conditions, but will be operating under both ATSAC and ATCS by year 2015. In accordance with City of Los Angeles procedures, a credit of 0.07 V/C reduction was applied at each intersection where ATSAC is implemented and an additional 0.03 V/C reduction was applied at each intersection where ATCS is implemented.

Existing Levels of Service

Existing traffic volumes were analyzed using the intersection capacity analysis methodology described above to determine the existing operating conditions at the two study intersections. Table 3 summarizes the results of the analysis of the Existing AM and PM peak hour V/C ratios and corresponding LOS. As indicated, both intersections operate at LOS A during both analyzed peak hours.

Existing plus Project Traffic Conditions

The project traffic presented in Figure 2 was added to existing traffic volumes to estimate Existing plus Project traffic volumes.

Existing plus Project Levels of Service

Existing plus Project traffic volumes were analyzed using the methodology described above to determine the operating conditions at the two study intersections with the addition of project traffic. Table 4 summarizes the results of the analysis of the Existing plus Project AM and PM peak hour V/C ratios and corresponding LOS. As indicated, both intersections operate at LOS A during both peak hours.

Future Base Traffic Conditions

To evaluate the potential impacts of the proposed project under future (Year 2015) conditions, it was necessary to develop estimates of future traffic conditions in the area both without and with project traffic. First, estimates of traffic growth were developed for the study area to forecast future conditions without the project. These projected traffic volumes, identified herein as the cumulative base conditions, represent the future year conditions without the proposed project.

Based on historic trends and at the direction of LADOT, it was established that a growth factor of 2% per year for a total of 4% for two year be applied to adjust the existing year traffic volumes to reflect the effects of local and regional growth in traffic by year 2015.

Additionally, per direction from LADOT, the full ATSAC/ATCS system installation will be complete in the area by year 2015; as such, a credit of 0.10 was applied to the study intersections under both future base and "plus project" conditions.



Future Base Levels of Service

Future base (Year 2015) traffic volumes were analyzed using the intersection capacity analysis methodology described above to determine the existing operating conditions at the two study intersections. Table 5 summarizes the results of the analysis of the Future Base AM and PM peak hour V/C ratios and corresponding LOS. As indicated, both intersections operate at LOS A during both peak hours.

Future plus Project Traffic Conditions

The project traffic presented in Figure 2 was added to future base traffic volumes to estimate future plus project conditions.

Future plus Project Levels of Service

Future plus Project traffic volumes were analyzed using the methodology described above to determine the existing operating conditions at the two study intersections. Table 6 summarizes the results of the analysis of the Future plus Project AM and PM peak hour V/C ratios and corresponding LOS. As indicated, both intersections operate at LOS A during both peak hours.

Impact Analysis

The traffic impact analysis compares the V/C ratios and LOS results at each study intersection under both existing and future conditions without and with the addition of project trips to estimate the incremental increase in the V/C ratio caused by the proposed project. This provides the information needed to assess the potential impact of the project using significance criteria established by LADOT.

Criteria for Determination of Significant Traffic Impact

The City of Los Angeles has established threshold criteria to determine significant traffic impact of a proposed Project in its jurisdiction. Under the LADOT guidelines, an intersection would be significantly impacted with an increase in V/C ratio equal to or greater than 0.04 for intersections operating at LOS C, equal to or greater than 0.02 for intersections operating at LOS D, and equal to or greater than 0.01 for intersections operating at LOS E or F after the addition of project traffic. Intersections operating at LOS A or B after the addition of the project traffic are not considered significantly impacted regardless of the increase in V/C ratio. The following summarizes the impact criteria:

LOS	Final V/C Ratio	Project Related Increase in V/C
С	>0.700 - 0.800	equal to or greater than 0.040
D	> 0.800 - 0.900	equal to or greater than 0.020
E or F	> 0.900	equal to or greater than 0.010



Existing plus Project Impact Analysis

As mentioned, all intersections operate at LOS A under both Existing and Existing plus Project conditions. As such, the project will not create any significant impacts at study intersections.

Future plus Project Impact Analysis

As mentioned, all intersections operate at LOS A under both Future and Future plus Project conditions. As such, the project will not create any significant impacts at study intersections.

NEIGHBORHOOD IMPACT ANALYSIS

In addition to Horizon Avenue and Market Place, the project site can be accessed using Toledo Court and turning onto the alley where the project driveway is located. Toledo Court functions as an alley providing access to garages and driveways for homes along this road. Toledo court is a narrow street with and intended for through traffic for land uses along the alley.

Traffic counts along Toledo Court were conducted during weekday evening peak period (4:00 to 6:00 PM) in December 2013. Vehicles using Toledo Court were counted to determine the number of "through" trips. Additionally, vehicles using the alley were counted to determine the number of "through" alley trips.

As shown in Table 7, Toledo Court and the adjacent alley have low peak hour traffic: 16 vehicles were observed using the two roadways over the course of two hours. Over that time, only one trip was a "through" trip on Toledo Court. As such, the project does not forecast that users will be accessing the site from Toledo Court, as users along this roadway were observed to solely be residents of Toledo Court.

SUMMARY

The 1414 Main Project plans to provide a mixed-use residential, restaurant, and retail development at the southeast corner of Horizon Avenue and Main Street. The project is estimated to generate a net total of 9 AM and 40 PM peak hour trips. The two closest signalized intersections operate at an acceptable LOS A during both peak hours and are forecasted to continue to operate at LOS A with the addition of project traffic under both Existing and Future Year (2015) conditions. Additionally, observations were held during the peak hour to determine whether Toledo Court would be a potential cut-through route. As observed, it is not anticipated that users would access the site via Toledo Court. As such, there are no significant impacts associated with the project.

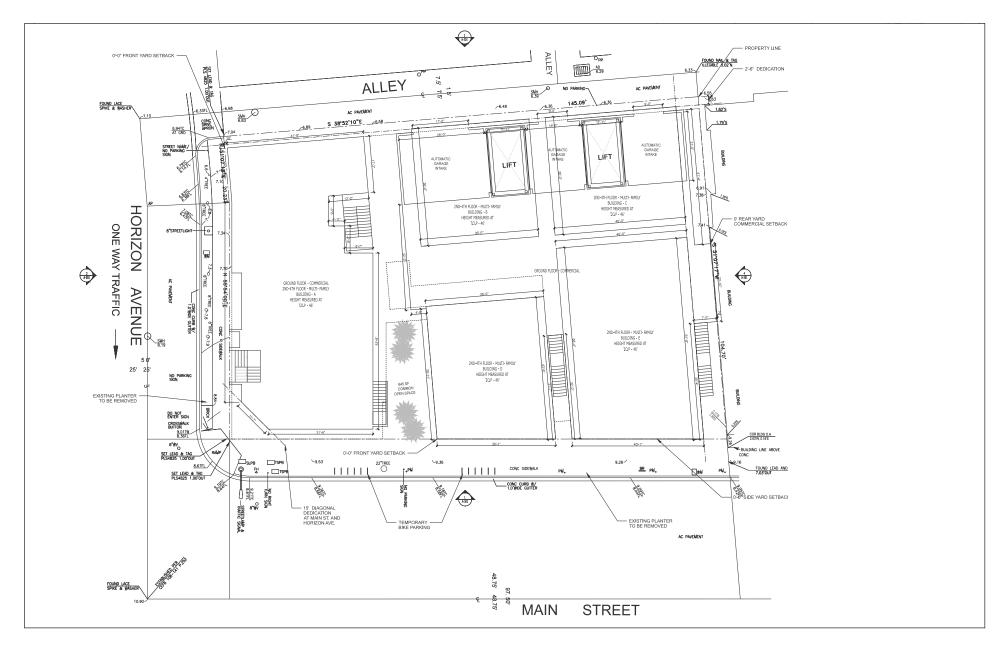












TABLE 1
TRIP GENERATION

		Daily	Daily	AM 1	Trip Genei	ration	PM 1	Trip Genei	ration		AM Trips			PM Trips	
Land Use	Size	Trip	Trips	Rate	In%	Out%	Rate	In%	Out%	Total	In	Out	Total	In	Out
Proposed Land Use 1,2													•		
Condominiums	26	5.81	151	0.44	17%	83%	0.7	67%	33%	11	2	9	18	12	6
Retail	1.184	42.7	51	0.96	62%	38%	14.6	48%	52%	1	1	0	17	8	9
Internal Capture ³			(5)							0	0	0	(2)	(1)	(1)
Pass-by ⁴			(23)							0	0	0	(7)	(3)	(4)
Net external retail			23							1	1	0	8	4	4
Restaurant	4.567	89.95	411	0.81	55%	45%	6.1	67%	33%	4	2	2	28	19	9
Internal Capture			(41)							0	0	0	(3)	(2)	(1)
Pass-by			(37)							0	0	0	(3)	(2)	(1)
Net external restaurant			333							4	2	2	22	15	7
Total			507							16	5	11	<mark>48</mark>	31	<mark>17</mark>
Existing Land Use															
Apartments	10	6.65	67	0.51	20%	80%	0.7	20%	80%	5	1	4	7	1	6
Single Family Residential	2	9.52	19	0.75	25%	75%	0.7	63%	37%	2	1	1	1	1	0
Total Existing Use Credit			86							7	2	5	8	2	6
Net New Trips			421							9	3	6	40	29	11

¹ Trip Generation (9 th Ed.), Institute of Transportation Engineers (ITE), 2012 was used for AM and Daily rates. Rate 230 for condominiums; 220 for apartments; 210 for single family dwelling units, rate 820 for retail, and rate 931 for restaurants.

² The Trip Generation rate from the Coastal Transportation Corridor Specific Plan (City of Los Angeles), 2004 was used for the PM peak hour for the proposed land use. The specific rate used was "shopping center less than 30,000 square feet" for retail and "low turnover" for restaurant, and "multi-story apartments, condominiums, townhomes or single-family housing" for apartments, condos, and single family residential.

³Internal capture represents the percentage of trips between land uses that occur within the site. This percentage is informed by the Multi-Use Trip Generation Calculation methodology described in Chapter 7 of the ITE *Trip Generation Handbook*, 2nd Edition, 2004. Internalization percentages are derived from NCHRP Project 8-51 as described in "Improved Estimation for Internal Capture for Mixed-Use Developments," *ITE Journal*, August 2010. For retail land use a 10% internal capture credit was applied.

⁴ Pass-by reduction of 50% per City of Los Angeles Traffic Study Guidelines (Los Angeles Department of Transportation, May 2012), Appendix H for Shopping Center less than 50,000 square feet for retail and 10% for restaurant.

TABLE 2 LEVEL OF SERVICE DEFINITIONS FOR SIGNALIZED INTERSECTIONS

Level of Service	Intersection Capacity Utilization	Definition
А	0.000-0.600	EXCELLENT. No Vehicle waits longer than one red
		light and no approach phase is fully used.
В	0.601-0.700	VERY GOOD. An occasional approach phase is
		fully utilized; many drivers begin to feel somewhat
		restricted within groups of vehicles.
С	0.701-0.800	GOOD. Occasionally drivers may have to wait
		through more than one red light; backups may
		develop behind turning vehicles.
D	0.801-0.900	FAIR. Delays may be substantial during portions
		of the rush hours, but enough lower volume periods
		occur to permit clearing of developing lines,
		preventing excessive backups.
E	0.901-1.000	POOR. Represents the most vehicles intersection
		approaches can accommodate; may be long lines
		of waiting vehicles through several signal cycles.
F	> 1.000	FAILURE. Backups from nearby locations or on
		cross streets may restrict or prevent movement of
		vehicles out of the intersection approaches.
		Tremendous delays with continuously increasing
		queue lengths.

Source: Transportation Research Circular No. 212, Interim Materials on Highway Capacity, Transportation Research Board, 1980.

TABLE 3
EXISTING (YEAR 2013) INTERSECTION LEVEL OF SERVICE ANALYSIS

Intersection	Peak Hour	Existing Base Year (Year 2013)				
	Houi	V/C	LOS			
1. Main Street	AM	0.093	A			
Horizon Avenue	PM	0.124	A			
2. Main Street	AM	0.450	A			
Abbot Kinney Blvd	PM	0.364	A			

Intersections analyzed using City of Los Angeles (CMA) methodology Intersections are currently operating under ATSAC system.

TABLE 4
EXISTING PLUS PROJECT INTERSECTION LEVEL OF SERVICE ANALYSIS

Intersection	Peak Hour	Existing Bas	e	Existing plus Project	S	Project Increase	Significant Project	
		V/C or Delay LOS		V/C or Delay LOS		in V/C	Impact	
1. Main Street	AM	0.093	A	0.094	A	0.001	NO	
Horizon Avenue	PM	0.124	A	0.130	A	0.006	NO	
2. Main Street	AM	0.450	A	0.451	A	0.001	NO	
Abbot Kinney Blvd	PM	0.364	A	0.367	A	0.003	NO	

Intersections analyzed using City of Los Angeles (CMA) methodology Intersections are currently operating under ATSAC system.

TABLE 5
FUTURE YEAR (YEAR 2015) INTERSECTION LEVEL OF SERVICE ANALYSIS

Intersection	Peak Hour	Future Year (Year 2015)				
	Houi	V/C	LOS			
1. Main Street	AM	0.074	A			
Horizon Avenue	PM	0.099	A			
2. Main Street	AM	0.441	A			
Abbot Kinney Blvd	PM	0.351	A			

Intersections analyzed using City of Los Angeles (CMA) methodology
Intersections are planned to operate under ATSAC and ATCS systems by year 2015.

TABLE 6
FUTURE YEAR (2015) PLUS PROJECT INTERSECTION LEVEL OF SERVICE ANALYSIS

Intersection	Peak Hour	Future Base	•	Future plus Project		Project Increase	Significant Project	
		V/C or Delay	LOS	V/C or Delay	LOS	in V/C	Impact	
Main Street Horizon Avenue	AM	0.074	A	0.075	A	0.001	NO	
	PM	0.099	A	0.107	A	0.008	NO	
Main Street Abbot Kinney Blvd	AM	0.441	A	0.442	A	0.001	NO	
	PM	0.351	A	0.354	A	0.003	NO	

Intersections analyzed using City of Los Angeles (CMA) methodology

Intersections are planned to operate under ATSAC and ATCS systems by year 2015.

TABLE 7
VEHICULAR TRAFFIC USING TOLEDO COURT
WEEKDAY EVENING PEAK HOURS (4:00-6:00 PM)

Origin	Destination	Used Toledo Court?	Used Alley?	Total Vehicles	Cut Through Trips	Local Trips	
Toledo Court (Eastbound)	Riviera Avenue (Northbound)	Yes	No	1	0	1	
Toledo Court (Eastbound)	Riviera Avenue (Southbound)	Yes	No	2	0	2	
Toledo Court (Westbound)	Market Street (Eastbound)	Yes	Yes	1	0	1	
Toledo Court (Westbound)	Horizon Avenue (Westbound)	Yes	Yes	3	0	3	
Alley (Northbound)	Horizon Avenue (Westbound)	No	Yes	2	0	2	
Alley (Southbound)	Market Street (Westbound)	No	Yes	1	0	1	
Riviera Avenue (Southbound)	Toledo Court (Eastbound)	Yes	No	1	0	1	
Riviera Avenue (Southbound)	Garage at "T" of Alley/Toledo Court	Yes	Yes	1	0	1	
Horizon Avenue (Westbound)	Toledo Court (Eastbound)	Yes	Yes	1	0	1	
Horizon Avenue (Westbound)	Market Street (Eastbound)	No	Yes	1	1	0	
Market Street (Eastbound)	Toledo Court (Eastbound)	Yes	Yes	2	0	2	
Total Vehicles Observed					16		
Total Trips Using Toledo Court					12		
Total Trips Using Alley			12				
Total Cut-Through Trips				1			
Total Local Trips		_		15			

ATTACHMENT A: TRAFFIC COUNTS

Intersection Turning Movement Prepared by:

National Data & Surveying Services

Project ID: 13-5586-001 **Day:** Tuesday **TOTALS**

City: Venice **Date:** 11/5/2013

_	AM												_
NS/EW Streets:		Main St			Main St		Abbott K	inney Blvd_ Ct	_Brooks	Abbott K	inney Blvd _. Ct	_Brooks	
	NO	ORTHBOUN	ID	SC	OUTHBOUN	ID	E	ASTBOUNI)	WESTBOUND			
LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 0	ET 1	ER 0	WL 0	WT 1	WR 1	TOTAL
7:00 AM 7:15 AM 7:30 AM 7:45 AM 8:00 AM 8:15 AM 8:30 AM 8:45 AM 9:00 AM 9:15 AM 9:30 AM	4 7 1 5 2 1 1 3 1 1 3 2	66 64 108 109 93 82 95 102 94 81 71 70	4 6 9 15 18 17 3 12 5 4 11 5	20 22 13 31 39 35 33 27 42 42 42 42	18 20 22 28 34 35 34 29 42 37 49 36	2 2 1 2 4 2 4 4 3 1	3 1 7 9 8 3 5 5 2 2 4 3	14 14 18 20 35 24 26 31 28 21 24 28	1 1 6 3 1 2 0 3 2 3 6	4 0 5 4 7 2 4 3 5 6 3	23 19 32 30 25 33 38 39 26 21 27 21	85 86 125 104 113 120 133 105 105 114 94 89	244 242 347 360 379 357 376 358 357 335 334 299
TOTAL VOLUMES : APPROACH %'s : PEAK HR START TIME :	NL 31 2.64%	NT 1035 88.09%	NR 109 9.28%	SL 381 47.98%	ST 384 48.36%	SR 29 3.65%	EL 52 14.21%	ET 283 77.32%	ER 31 8.47%	WL 46 2.78%	WT 334 20.21%	WR 1273 77.01%	TOTAL 3988
PEAK HR START TIME : PEAK HR VOL : PEAK HR FACTOR :	9	379 0.855	53	138	131 0.930	10	25	105 0.755	9	17	126 0.876	470	1472 0.971

Intersection Turning Movement

Prepared by:

National Data & Surveying Services

Project ID: 13-5586-001

TOTALS

Day: Tuesday

City: Venice Date: 11/5/2013

_	PM												
NS/EW Streets:		Main St		Main St			Abbott Kinney Blvd_Brooks Ct			Abbott Kinney Blvd_Brooks Ct			
	NO	ORTHBOU	ND	SC	OUTHBOUN	ID	E	ASTBOUN	D	WESTBOUND			
LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 0	ET 1	ER 0	WL 0	WT 1	WR 1	TOTAL
3:00 PM 3:15 PM 3:30 PM 3:45 PM 4:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM	2 3 4 2 1 1 1 1 5 2	56 58 42 57 56 45 55 42 52 52 53	15 9 7 6 7 8 12 7 9	59 71 70 63 79 82 79 66 72 78 71	73 64 84 112 90 105 113 92 114 114 124	3 3 2 2 5 3 5 0 4 3	4 6 1 3 3 3 3 2 4 6 3	23 20 38 37 50 43 36 43 41 48 34	3 8 3 6 4 4 4 2 2 7 7	10 3 10 10 6 11 8 4 5 8	29 21 20 26 30 24 19 25 20 22 32	61 64 75 66 59 58 63 64 57 62 73	338 330 357 390 387 389 396 353 381 412 419
5:45 PM	0	53	5	73	121	1	6	38	6	12	17	56	388
TOTAL VOLUMES : APPROACH %'s :	NL 24 3.22%	NT 621 83.24%	NR 101 13.54%	SL 863 41.04%	ST 1206 57.35%	SR 34 1.62%	EL 44 7.99%	ET 451 81.85%	ER 56 10.16%	WL 97 8.51%	WT 285 25.00%	WR 758 66.49%	TOTAL 4540
PEAK HR START TIME:	500	PM											TOTAL
PEAK HR VOL:	9	210	30	294	473	8	19	161	22	35	91	248	1600
PEAK HR FACTOR:		0.943			0.979			0.828			0.813		0.955

PREPARED BY NATIONAL DATA & SURVEYING SERVICES

PROJECT#: 13-5586-001 N/S Street: Main St

E/W Street: Abbott Kinney Blvd_Brooks Ct

DATE: 11/5/2013 DAY: Tuesday CITY: Venice

A M

Adult Pedestrians

Auuit Peuest	ilalis							
TIME	NORT	H LEG	SOUT	H LEG	EAST	LEG	WES	Γ LEG
1 M L	EB	WB	EB	WB	NB	SB	NB	SB
7:00 AM	0	0	5	13	2	5	2	1
7:15 AM	1	0	5	4	1	3	1	0
7:30 AM	3	1	4	5	1	4	1	3
7:45 AM	0	0	6	9	4	7	2	0
8:00 AM	0	0	10	6	4	9	3	1
8:15 AM	1	0	6	11	4	9	3	2
8:30 AM	1	0	6	10	8	6	5	0
8:45 AM	0	0	6	4	4	3	1	3
9:00 AM	0	1	7	2	5	3	3	2
9:15 AM	0	1	2	9	5	6	1	1
9:30 AM	3	0	7	2	7	3	1	4
9:45 AM	0	0	7	7	10	6	1	2
TOTALS	9	3	71	82	55	64	24	19

School-Aged Pedestrians

School-Agea		H LEG	COLIT	H LEG	EVCI	LEG	WEC	Γ LEG
TIME								
	EB	WB	EB	WB	NB	SB	NB	SB
7:00 AM	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	1	0	0
7:45 AM	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	1	0	0
8:30 AM	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0
9:00 AM	0	0	0	0	0	0	0	0
9:15 AM	0	0	0	0	0	0	0	0
9:30 AM	0	0	0	0	0	0	0	0
9:45 AM	0	0	0	0	0	0	0	0
TOTALC	^	^	_	^		2	^	^

PM

Adult Pedestrians NORTH LEG | SOUTH LEG EAST LEG WEST LEG TIME NB EB WB EB WB NB SB SB 3:00 PM 3:15 PM 3:30 PM 3:45 PM 4:00 PM 4:15 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM **TOTALS**

School-Aged Pedestrians

etheth riged								
TIME	NORT	H LEG	SOUT	H LEG	EAST	LEG	WES	T LEG
IIIVIE	EB	WB	EB	WB	NB	SB	NB	SB
3:00 PM	0	0	0	0	0	0	0	0
3:15 PM	0	0	0	1	0	0	0	0
3:30 PM	0	0	0	0	0	0	0	0
3:45 PM	0	2	0	0	0	0	0	0
4:00 PM	0	0	0	2	0	0	0	0
4:15 PM	0	0	0	0	3	0	0	0
4:30 PM	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0
TOTALS	0	2	0	3	3	0	0	0

Intersection Turning Movement

Prepared by:

National Data & Surveying Services

Project ID: 13-5586-001

BIKES

Day: Tuesday

City: Venice **Date:** 11/5/2013

City: \						Al	М				Duto.	11/3/2010	•
NS/EW Streets:		Main St			Main St			inney Blvd_ Ct	_Brooks	Abbott K	inney Blvd Ct	_Brooks	
	NO	ORTHBOUN	ND	SC	DUTHBOUN	ID	E	ASTBOUNI)	V	/ESTBOUN	ID	
LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 0	ET 1	ER 0	WL 0	WT 1	WR 1	TOTAL
7:00 AM	0	3	1	1	1	0	0	2	0	1	1	2	12
7:15 AM	0	5	1	0	0	0	0	0	0	1	2	4	13
7:30 AM	0	5	1	6	1	0	0	1	0	1	2	5	22
7:45 AM	0	12	1	2	3	1	0	3	0	0	2	5	29
8:00 AM	0	5	1	1	3	0	0	3	0	0	4	4	21
8:15 AM	0	6	0	1	4	3	0	3	0	1	3	6	27
8:30 AM	0	10	2	0	1	0	0	3	0	0	3	5	24
8:45 AM	0	15	1	1	7	0	0	0	0	2	1	4	31
9:00 AM	0	9	2	2	5	0	0	0	0	0	7	4	29
9:15 AM	0	8	0	1	2	0	1	4	0	1	0	2	19
9:30 AM	0	9	2	5	5	0	1	3	0	1	3	0	29
9:45 AM	0	25	0	3	2	0	0	1	0	0	3	5	39
	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
TOTAL VOLUMES:	0	112	12	23	34	4	2	23	0	8	31	46	295
APPROACH %'s:	0.00%	90.32%	9.68%	37.70%	55.74%	6.56%	8.00%	92.00%	0.00%	9.41%	36.47%	54.12%	l
PEAK HR START TIME :	745	AM											TOTAL
PEAK HR VOL :	0	33	4	4	11	4	0	12	0	1	12	20	101
PEAK HR FACTOR:		0.712			0.594			1.000			0.825		0.871

Intersection Turning Movement

Prepared by:

National Data & Surveying Services

Project ID: 13-5586-001

BIKES

Day: Tuesday

City: Venice **Date:** 11/5/2013

Oity.	Vernee					PI	М				Date	11/5/2015	,
NS/EW Streets:		Main St			Main St		Abbott K	inney Blvd Ct	_Brooks	Abbott K	inney Blvd Ct	_Brooks	
	NO	ORTHBOU	ND	SC	DUTHBOUN	ND	E	ASTBOUN	D	V	VESTBOUN	ID	
LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 0	ET 1	ER 0	WL 0	WT 1	WR 1	TOTAL
3:00 PM	0	2	1	7	0	1	0	1	0	0	0	3	15
3:15 PM 3:30 PM	0 0	3 4	0	2	4 3	0 0	0 2	1	0	2	4 2	1 0	17 21
3:45 PM	0	4	0	1	4	0	0	2	1	0	1	1	14
4:00 PM	0	3	0	0	3	0	0	6	2	2	0	2	18
4:15 PM	1	4	1	3	4	1	1	5	1	3	1	3	28
4:30 PM	1	6	1	2	6	0	0	3	0	0	1	1	21
4:45 PM	1	4	1	4	8	0	0	0	0	0	9	4	31
5:00 PM	0	5	1	3	7	0	0	7	1	2	3	0	29
5:15 PM	0	13	0	0	6	0	0	5	1	1	6	/	39
5:30 PM 5:45 PM	13	0 0	6 4	0	0 0	0	3 1	0 0	2	3 8	0 0	/ 2	29 32
3. 1 3 FM	13	U	7	U	U	1	1	U	3	0	U	2	32
TOTAL VOLUMES : APPROACH %'s :	NL 23 26.44%	NT 48 55.17%	NR 16 18.39%	SL 30 38.46%	ST 45 57.69%	SR 3 3.85%	EL 7 14.29%	ET 31 63.27%	ER 11 22.45%	WL 22 27.50%	WT 27 33.75%	WR 31 38.75%	TOTAL 294
PEAK HR START TIME :	500	PM											TOTAL
PEAK HR VOL :	20	18	11	4	13	1	4	12	7	14	9	16	129
PEAK HR FACTOR :		0.721			0.450			0.719			0.696		0.827

Intersection Turning Movement Prepared by:

National Data & Surveying Services

Project ID: 13-5586-002 **Day:** Tuesday **TOTALS**

City: Venice **Date:** 11/5/2013

Oity: V	Cincc					A	М				Dutci	11/3/2013	•
NS/EW Streets:		Main St			Main St		H	Horizon Ave	e	Н	orizon Ave	2	
	NO	ORTHBOUN	ND	SC	OUTHBOUN	ID		EASTBOUN	ID	W	ESTBOUN	ID	
LANES:	NL 1	NT 2	NR 0	SL 0	ST 2	SR 0	EL 0	ET 0	ER 0	WL 0	WT 1	WR 0	TOTAL
7:00 AM	1	62	0	0	21	1	0	0	0	0	0	2	87
7:15 AM	2	65	0	0	19	0	0	0	0	1	0	3	90
7:30 AM	0	98	0	0	25	3 5	0	0	0	1	1	3	131
7:45 AM 8:00 AM	4 0	105 75	0	0 0	35 37	5 4	0 0	0 0	0 0	1	0	7	152 124
8:15 AM	7	96	0	0	37 37	3	0	0	0	3	0	1	147
8:30 AM	6	93	0	0	42	4	0	0	0	3	1	4	153
8:45 AM	4	98	0	0	37	5	0	0	0	0	1	7	152
9:00 AM	1	84	0	0	47	3	0	0	0	1	1	7	144
9:15 AM	6	78	0	0	43	5	0	0	0	1	1	5	139
9:30 AM	2	79	0	0	57	8	0	0	0	3	0	7	156
9:45 AM	4	61	0	0	45	5	0	0	0	0	0	5	120
TOTAL VOLUMES : APPROACH %'s :	NL 37 3.59%	NT 994 96.41%	NR 0 0.00%	SL 0 0.00%	ST 445 90.63%	SR 46 9.37%	EL 0 #DIV/0!	ET 0 #DIV/0!	ER 0 #DIV/0!	WL 15 20.55%	WT 6 8.22%	WR 52 71.23%	TOTAL 1595
PEAK HR START TIME :	815	AM											TOTAL
PEAK HR VOL:	18	371	0	0	163	15	0	0	0	7	3	19	596
PEAK HR FACTOR:		0.944			0.890			0.000			0.806		0.974

Intersection Turning Movement

Prepared by:

National Data & Surveying Services

Project ID: 13-5586-002

TOTALS

Day: Tuesday

City: Venice **Date:** 11/5/2013

_						P	M						•
NS/EW Streets:		Main St			Main St		ŀ	Horizon Ave	e	Н	orizon Ave	2	
	NO	ORTHBOUN	ND	SC	OUTHBOUN	ID	I	EASTBOUN	ID	W	/ESTBOUN	D	
LANES:	NL 1	NT 2	NR 0	SL 0	ST 2	SR 0	EL 0	ET 0	ER 0	WL 0	WT 1	WR 0	TOTAL
3:00 PM	6	63	0	0	93	4	0	0	0	0	0	1	167
3:15 PM	3	50	0	0	93 81	3	0	0	0	2	0	2	141
3:30 PM	3	43	0	0	91	3 4	0	0	0	0	1	2	141 144
3:45 PM	2	5 7	0	0	117	2	0	0	0	1	0	1	180
4:00 PM	4	49	0	0	98	5	0	0	0	0	0	1	157
4:15 PM	1	50	0	0	119	2	0	0	0	0	0	2	174
4:30 PM	1	48	0	0	126	5	0	0	0	2	3	1	186
4:45 PM	2	48	0	0	102	4	0	0	0	1	1	1	159
5:00 PM	1	62	0	0	119	3	0	0	0	2	0	2	189
5:15 PM	3	42	0	0	123	3	0	0	0	1	1	0	173
5:30 PM	3	47	0	0	138	2	0	0	0	0	2	3	195
5:45 PM	5	49	0	0	135	1	0	0	0	1	0	1	192
3113111		13			155	-				-		-	132
	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
TOTAL VOLUMES:	34	608	0	0	1342	38	0	0	0	10	8	17	2057
APPROACH %'s:	5.30%	94.70%	0.00%	0.00%	97.25%	2.75%	#DIV/0!	#DIV/0!	#DIV/0!	28.57%	22.86%	48.57%	
PEAK HR START TIME :	500	PM											TOTAL
													. 0 . /
PEAK HR VOL:	12	200	0	0	515	9	0	0	0	4	3	6	749
PEAK HR FACTOR:		0.841			0.936			0.000			0.650		0.960

PREPARED BY NATIONAL DATA & SURVEYING SERVICES

DAY:

PROJECT#: 13-5586-002 N/S Street: Main St E/W Street: Horizon Ave DATE: 11/5/2013

CITY: Venice

A M

Adult Pedestrians

TIME	NORT	H LEG	SOUT	H LEG	EAST	LEG	WES	ΓLEG
TIME	EB	WB	EB	WB	NB	SB	NB	SB
7:00 AM	0	2	0	2	3	1	0	2
7:15 AM	3	1	0	3	2	3	5	2
7:30 AM	1	1	0	1	3	0	1	1
7:45 AM	2	2	2	1	4	0	1	1
8:00 AM	1	2	0	0	4	1	1	4
8:15 AM	2	0	0	1	1	0	5	4
8:30 AM	2	3	0	1	2	1	3	3
8:45 AM	0	2	0	2	2	3	1	6
9:00 AM	0	1	2	3	0	1	2	4
9:15 AM	0	0	0	4	6	3	1	2
9:30 AM	1	0	2	3	4	3	2	2
9:45 AM	2	3	2	4	1	3	1	5
TOTALS	14	17	8	25	32	19	23	36

School-Aged Pedestrians

Tuesday

TIME	NORT	H LEG	SOUT	H LEG	EAST	LEG	WES	Γ LEG
TIME	EB	WB	EB	WB	NB	SB	NB	SB
7:00 AM	0	0	0	0	0	0	0	0
7:15 AM	1	0	0	0	0	0	1	0
7:30 AM	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0
9:00 AM	0	0	0	0	0	0	0	0
9:15 AM	0	0	0	0	0	0	0	0
9:30 AM	0	0	0	0	0	0	0	0
9:45 AM	0	0	0	0	0	0	0	0
TOTALS	1	0	0	0	0	0	1	0

P M

Adult Pedestrians

TIME	NORT	H LEG	SOUT	H LEG	EAST	LEG	WES	T LEG
11141⊏	EB	WB	EB	WB	NB	SB	NB	SB
3:00 PM	1	1	3	1	2	3	2	5
3:15 PM	1	3	0	2	3	3	6	2
3:30 PM	5	2	2	3	4	7	4	4
3:45 PM	4	2	1	0	2	7	3	6
4:00 PM	2	0	6	7	1	6	5	3
4:15 PM	2	1	0	2	13	1	4	5
4:30 PM	3	1	3	0	2	5	5	4
4:45 PM	4	5	1	3	1	1	7	7
5:00 PM	5	2	3	1	0	1	3	6
5:15 PM	5	3	9	2	5	7	7	9
5:30 PM	1	0	6	3	9	3	3	2
5:45 PM	3	2	20	0	3	2	5	2
TOTALS	36	22	54	24	45	46	54	55

School-Aged Pedestrians

TIME	NORT	H LEG	SOUT	H LEG	EAST	LEG	WES	T LEG
1114 ⊏	EB	WB	EB	WB	NB	SB	NB	SB
3:00 PM	0	0	0	0	0	1	0	0
3:15 PM	0	0	0	0	0	0	0	0
3:30 PM	0	0	0	4	0	4	0	0
3:45 PM	0	0	0	0	0	0	0	0
4:00 PM	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0
5:15 PM	0	1	0	0	0	0	0	1
5:30 PM	0	0	0	0	0	1	0	0
5:45 PM	0	0	0	0	0	0	0	0
TOTALS	0	1	0	4	0	6	0	1

Intersection Turning Movement Prepared by:

National Data & Surveying Services

Project ID: 13-5586-002 **Day:** Tuesday **BIKES**

City: Venice **Date:** 11/5/2013

Gity!						A	М				2433.	11/3/2013	
NS/EW Streets:		Main St			Main St		Н	orizon Ave		Н	orizon Ave	2	
	NO	ORTHBOUN	ND	SC	DUTHBOU	ND	E	ASTBOUNI)	V	VESTBOUN	ID	
LANES:	NL 1	NT 2	NR 0	SL 0	ST 2	SR 0	EL 0	ET 0	ER 0	WL 0	WT 1	WR 0	TOTAL
7:00 AM	0	7	0	0	2	0	0	0	0	0	0	0	9
7:15 AM	0	3	0	0	1	0	0	0	0	0	0	0	4
7:30 AM	0	8	0	0	1	1	0	0	0	1	0	0	11
7:45 AM	0	12	0	0	3	2	1	0	0	0	0	0	18
8:00 AM	0	3	0	0	3	0	0	0	0	0	0	1	7
8:15 AM	0	7	0	0	3	0	0	1	0	0	2	0	13
8:30 AM	0	14	0	0	4	0	0	0	0	0	0	0	18
8:45 AM	0	12	0	1	9	1	0	0	0	0	0	0	23
9:00 AM	0	10	0	0	3	0	0	0	0	1	0	1	15
9:15 AM	0	10	0	0	3	1	0	0	0	0	0	0	14
9:30 AM	0	12	0	0	2	1	0	2	0	0	0	0	17
9:45 AM	1	26	0	0	4	2	0	0	0	0	0	0	33
	NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL
TOTAL VOLUMES : APPROACH %'s :	1 0.80%	124 99.20%	0 0.00%	1 2.13%	38 80.85%	8 17.02%	1 25.00%	3 75.00%	0 0.00%	2 33.33%	2 33.33%	2 33.33%	182
PEAK HR START TIME :	815	AM											TOTAL
PEAK HR VOL:	0	43	0	1	19	1	0	1	0	1	2	1	69
PEAK HR FACTOR:		0.768			0.477			0.250			0.500		0.750

Intersection Turning Movement

Prepared by:

National Data & Surveying Services

Project ID: 13-5586-002

BIKES

Day: Tuesday

City: Venice **Date:** 11/5/2013

	Vernee					PI	М				Date	11/3/2013	
NS/EW Streets:		Main St			Main St		Н	lorizon Ave	2	Н	lorizon Ave	2	
	N	ORTHBOU	ND	SC	OUTHBOUN	ID	E	ASTBOUN	D	V	VESTBOUN	ID	
LANES:	NL 1	NT 2	NR 0	SL 0	ST 2	SR 0	EL 0	ET 0	ER 0	WL 0	WT 1	WR 0	TOTAL
3:00 PM	0	3	2	0	2	0	0	0	0	0	0	0	7
3:15 PM 3:30 PM	0 0	3 2	3 1	0 0	4 5	0 0	0	0 1	0 0	0 1	0 3	0 0	11 13
3:45 PM 4:00 PM	0 0	5 3	1 0	0 0	4 4	0 0	1 0	0 0	0 0	0 0	1 0	0	12 7
4:15 PM 4:30 PM	0	4 5	0	0	4	0	0	0	0	0	0	0	8 15
4:45 PM	0	7	0	0	8	3	0	0	1	0	0	0	19
5:00 PM 5:15 PM	0 0	12 10	0 1	0 0	11 7	0 0	0 0	0 0	0 0	0	0 1	0 1	23 20
5:30 PM 5:45 PM	0 0	8 6	0 1	3 5	5 4	1 0	1 2	5 3	0 1	0 0	2 1	2 1	27 24
TOTAL VOLUMES : APPROACH %'s :	NL 0 0.00%	NT 68 87.18%	NR 10 12.82%	SL 8 10.53%	ST 64 84.21%	SR 4 5.26%	EL 5 31.25%	ET 9 56.25%	ER 2 12.50%	WL 3 18.75%	WT 8 50.00%	WR 5 31.25%	TOTAL 186
PEAK HR START TIME :	500	PM											TOTAL
PEAK HR VOL:	0	36	2	8	27	1	3	8	1	0	4	4	94
PEAK HR FACTOR:		0.792			0.818			0.500			0.500		0.870

ATTACHMENT B: LOS WORKSHEETS





I/S #:

PROJECT TITLE:

North-South Street: Main Street East-West Street: Horizon Ave

Scenario: EXISTING CONDITIONS

Count Date: Analyst: Date:

			АМ			PM	
	No. of Phases			2			2
	Opposed Ø'ing: N/S-1, E/W-2 or Both-3?			0			0
l i	Right Turns: FREE-1, NRTOR-2 or OLA-3?	NB 0	SB	0	NB 0	SB	0
		EB 0	WB	0	EB 0	WB	0
	ATSAC-1 or ATSAC+ATCS-2? Override Capacity			1			1 0
	Override Capacity		No. of	Lane		No. of	Lane
	MOVEMENT	Volume	Lanes	Volume	Volume	Lanes	Volume
	↑ Left	18	1	18	12	1	12
N	← Left-Through		0		-	0	
nc	↑ Through	371	1	186	200	1	100
P P	↑ Through-Right		1			1	
¥.	Right	0	0	0	0	0	0
NORTHBOUND	← Left-Through-Right		0			0	
_	Left-Right		0			0	
					,		
Q	← Left	0	0	0	0	0	0
S	⇒ Left-Through		1			1	
ВО	Through	163	0	89	515	0	262
SOUTHBOUND	✓ Through-Right✓ Right	15	1 0	89	9	0	262
DC.	← Left-Through-Right	15	0	09	9	0	202
SC	Left-Right		0			0	
	24 -011g				1	·	
_	ے Left	0	0	0	0	0	0
N	→ Left-Through		0			0	
OO	→ Through	0	0	0	0	0	0
TB	→ Through-Right		0		_	0	
EASTBOUND	Right	0	0	0	0	0	0
E,	★ Left-Through-Right ★ Left-Right		0 0			0 0	
			U				
	√ Left	7	0	7	4	0	4
2		,	0	,		0	ľ
ESTBOUND	← Through	3	0	29	3	0	13
 B	Through-Right		0			0	
ES:	Right	19	0	0	6	0	0
WE	Left-Through-Right		1			1	
	├─ Left-Right		0	100		0	07.4
	CRITICAL VOLUMES	Λ	orth-South:	186		lorth-South:	274
	CHITICAL VOLUMES		East-West: SUM:	29 215		East-West: SUM:	13 287
	VOLUME/CAPACITY (V/C) RATIO:		SUIVI:			SUIVI:	
	• • •			0.143			0.191
<i>V</i> /	C LESS ATSAC/ATCS ADJUSTMENT:			0.093			0.124
	LEVEL OF SERVICE (LOS):			Α			Α





I/S #: 2 PROJECT TITLE:

North-South Street: Main Street East-West Street: Abbot Kinney

Scenario: EXISTING CONDITIONS

Count Date: Analyst: Date:

AM PM							
	No. of Phases			2			2
	Opposed Ø'ing: N/S-1, E/W-2 or Both-3?			0			0
	Right Turns: FREE-1, NRTOR-2 or OLA-3?	NB 0	SB	0	NB 0	SB	0
		EB 0	WB	0	EB 0	WB	0
	ATSAC-1 or ATSAC+ATCS-2?			1			1 0
	Override Capacity		No. of	Lane		No. of	Lane
	MOVEMENT	Volume	Lanes	Volume	Volume	Lanes	Volume
	↑ Left	9	1	9	9	1	9
9	← Left-Through	J	0	J		0	J
Ď	↑ Through	379	1	216	210	1	120
BG	↑ Through-Right	070	1	210	210	1	120
l E	Right	53	0	53	30	0	30
NORTHBOUND	← Left-Through-Right		0	50		0	- 00
Ž	Left-Right		0			0	
			J				
	- ← Left	138	1	138	294	1	294
SOUTHBOUND	Left-Through		0	.00		0	
□ 0	↓ Through	131	1	71	473	1	241
4	← Through-Right		1			1	
Ē	ب Right َ	10	0	10	8	0	8
Į į	← Left-Through-Right		0			0	
S	← Left-Right		0			0	
			-		_		
EASTBOUND		25	0	25	19	0	19
	→ Left-Through		0	400		0	
i တွ	→ Through	105	0	139	161	0	202
H E	→ Through-Right	0	0 0	0	22	0 0	0
AS	Right Left-Through-Right	9	1	0	22	1	0
Ш	→ Left-Tillough-Right		0			0	
			J			U U	
	√ Left	17	0	17	35	0	35
Ş			1			1	
■ ⊼	← Through	126	0	143	91	0	126
ESTBOUND	← Through-Right		0			0	
ST	Right	470	1	401	248	1	101
WE	Left-Through-Right		0			0	
	├─ Left-Right		0			0	
		۸	lorth-South:		٨	lorth-South:	414
	CRITICAL VOLUMES		East-West:	426		East-West:	237
			SUM:	780		SUM:	651
	VOLUME/CAPACITY (V/C) RATIO:			0.520			0.434
V/	C LESS ATSAC/ATCS ADJUSTMENT:			0.450			0.364
	LEVEL OF SERVICE (LOS):			A			A
<u> </u>	22122 31 3211132 (200).						^





I/S #:

PROJECT TITLE:

North-South Street: Main Street East-West Street: Horizon Ave

Scenario: EXISTING + PROJECT CONDITIONS

Count Date: Analyst: Date:

AM PM							
	No. of Phases			2			2
	Opposed Ø'ing: N/S-1, E/W-2 or Both-3?			0			0
	Right Turns: FREE-1, NRTOR-2 or OLA-3?	NB 0	SB	0	NB 0	SB	0
	ATSAC-1 or ATSAC+ATCS-2?	<i>EB</i> 0	WB	0 1	EB 0	WB	0
	Override Capacity			0			0
			No. of	Lane		No. of	Lane
	MOVEMENT	Volume	Lanes	Volume	Volume	Lanes	Volume
	↑ Left	18	1	18	12	1	12
Į	← Left-Through		0			0	
NORTHBOUND	↑ Through	371	1	186	200	1	100
l ≝	→ Through-Right		1			1	
l Æ	Right	0	0	0	0	0	0
S	Left-Through-Right		0			0	
	Left-Right		0			0	
	√ Left	0	0	0	0	0	0
SOUTHBOUND	Left-Through		1	J		1	Ũ
O	↓ Through	164	0	90	527	0	268
띺	← Through-Right		1			1	
5	→ Right	15	0	90	9	0	268
90	← Left-Through-Right		0			0	
.	∠ Left-Right	l .	0			0	
		0	0	0	0	0	0
₽	→ Left-Through	Ů	0	U		0	U
EASTBOUND	→ Through	0	0	0	0	0	0
B	→ Through-Right		0			0	
ST	Right	0	0	0	0	0	0
E/	Left-Through-Right		0			0	
	Left-Right	<u> </u>	0			0	
	√ Left	8	0	8	6	0	6
9	√ Left-Through		0	J		0	U
Į	← Through	3	0	32	3	0	19
<u> </u>	Through-Right		0			0	
ESTBOUND	Right	21	0	0	10	0	0
WE	Left-Through-Right		1			1	
	├─ Left-Right		O	100		O	000
	CRITICAL VOLUMES	^	lorth-South: East-West:	186 32	_ ^	lorth-South: East-West:	280 19
	CHITICAL VOLUMES		East-West: SUM:	32 218		East-West: SUM:	299
	VOLUME/CAPACITY (V/C) RATIO:					JOIN.	
17	/C LESS ATSAC/ATCS ADJUSTMENT:			0.145			0.199
"				0.094			0.130
<u> </u>	LEVEL OF SERVICE (LOS):			Α			Α





I/S #:

PROJECT TITLE:

North-South Street: Main Street East-West Street: Abbot Kinney

Scenario: EXISTING + PROJECT CONDITIONS

Count Date: Analyst: Date:

No. of Phases Opposed Ø'ing: N/S-1, E/W-2 or Both-3? Right Turns: FREE-1, NRTOR-2 or OLA-3? ATSAC-1 or ATSAC+ATCS-2? Override Capacity Override Capacity	2 0 0 0
Right Turns: FREE-1, NRTOR-2 or OLA-3? NB 0 SB 0 NB 0 N	0 0
ATSAC-1 or ATSAC+ATCS-2?	0
ATSAC-1 or ATSAC+ATCS-2?	
Override Capacity MOVEMENT Volume No. of Lane Volume Lane Volume No. of Lane Volume No. of Lane Volume No. of Lane Volume Lanes L	- 1
MOVEMENT Volume No. of Lane Volume Volume Lanes	0
Novement Volume Lanes Volume Volume Lanes	
QND QH Left-Through 9 1 9 10 1 Left-Through 381 1 217 213 1 Through-Right 1 1 1 1 Right 53 0 53 30 0 Left-Through-Right 0 0 0 0 Left-Right 0 0 0 0	-
Continue Continue	10
Left-Right 0 0 0	
Left-Right 0 0 0	122
Left-Right 0 0 0	
Left-Right 0 0 0	30
Left-Right 0 0 0	
Q DO	
C C C C C C C C C C	
S	294
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	
〒 ← Inrougn-Right	245
	0
5	8
o Left-Right 0	
Lett-riight	
J Left 25 0 25 19 0	19
Q	
QND ON DOWN CONTROL → Left-Through 0	205
m	
9 0 0 25 0	0
_	25
Q	35
2	126
Om through 120 0 143 91 0	.20
QN DO MED Through 1 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	101
Left-Through-Right 0	
> Left-Right 0	
North-South: 355 North-South:	416
CRITICAL VOLUMES East-West: 426 East-West:	240
SUM: 781 SUM:	656
VOLUME/CAPACITY (V/C) RATIO: 0.521	437
V/C LESS ATSAC/ATCS ADJUSTMENT: 0.451	367
LEVEL OF SERVICE (LOS):	





I/S #:

PROJECT TITLE:

North-South Street: Main Street East-West Street: Horizon Ave

Scenario: CUMULATIVE BASE

Count Date: Analyst: Date:

AM PM							
	No. of Phases			2			2
	Opposed Ø'ing: N/S-1, E/W-2 or Both-3?			0			0
	Right Turns: FREE-1, NRTOR-2 or OLA-3?	NB 0	SB	0	NB 0	SB	0
	ATSAC-1 or ATSAC+ATCS-2?	EB 0	WB	0 2	EB 0	WB	0 2
	Override Capacity			0			0
			No. of	Lane		No. of	Lane
	MOVEMENT	Volume	Lanes	Volume	Volume	Lanes	Volume
	↑ Left	19	1	19	12	1	12
ĮĘ	← Left-Through		0			0	
NORTHBOUND	↑ Through	386	1	193	208	1	104
l ≝	→ Through-Right		1			1	
F	Right	0	0	0	0	0	0
2	Left-Through-Right		0			0	
	Left-Right		0			0	
	∀ Left	0	0	0	0	0	0
SOUTHBOUND	Left-Through	J	1	J		1	U
0	↓ Through	170	0	93	536	0	273
Ψ	← Through-Right		1			1	
E	ب Right	16	0	93	9	0	273
Į į	← Left-Through-Right		0			0	
٠,			0			0	
	J Left	0	0	0	0	0	0
₽	→ Left-Through	U	0	U	U	0	U
EASTBOUND	→ Through	0	0	0	0	0	0
BO	→ Through-Right		0			0	-
ST	Right	0	0	0	0	0	0
EA	Left-Through-Right		0			0	
	-		0			0	
	Cloft	7	0	7		0	1
₽	✓ Left ✓ Left-Through	7	0 0	7	4	0 0	4
ESTBOUND	← Through	3	0	30	3	0	13
B0	Through-Right		0			0	.0
EST	Right	20	0	0	6	0	0
WE	Left-Through-Right		1			1	
	├─ Left-Right		0			0	
	ODITIOAL VOLUMES	North-South:		193	North-South:		285
	CRITICAL VOLUMES		East-West:	30		East-West:	13
	VOLUME/CAPACITY (V/C) RATIO:		SUM:	223		SUM:	298
				0.149			0.199
<i>V</i>	/C LESS ATSAC/ATCS ADJUSTMENT:			0.074			0.099
	LEVEL OF SERVICE (LOS):			Α			Α





I/S #: 2 PROJECT TITLE:

North-South Street: Main Street East-West Street: Abbot Kinney

Scenario: CUMULATIVE BASE

Count Date: Analyst: Date:

AM PM							
	No. of Phases			2			2
	Opposed Ø'ing: N/S-1, E/W-2 or Both-3?			0			0
	Right Turns: FREE-1, NRTOR-2 or OLA-3?	NB 0	SB	0	NB 0	SB	0
		EB 0	WB	0	EB 0	WB	0
	ATSAC-1 or ATSAC+ATCS-2?			2			2
	Override Capacity		No. of	Lane		No. of	Lane
	MOVEMENT	Volume	Lanes	Volume	Volume	Lanes	Volume
	↑ Left	9	1	9	9	1	9
N	← Left-Through		0			0	
DC	↑ Through	394	1	225	218	1	125
1B(⇒ Through-Right		1			1	
₹.	Right	55	0	55	31	0	31
NORTHBOUND	← Left-Through-Right		0			0	
_	Left-Right		0			0	
					<u>, </u>		
Ω	t ← Left	144	1	144	306	1	306
SOUTHBOUND	⇒ Left-Through		0			0	0.50
30	Through	136	1	73	492	1	250
Ĕ	← Through-Right	10	1	10		1	0
Ď		10	0 0	10	8	0 0	8
SC	Left-Right		0			0	
	Zoternight						
	→ Left	26	0	26	20	0	20
PD	→ Left-Through		0			0	
חכ	→ Through	109	0	144	167	0	210
EASTBOUND	◯ Through-Right		0			0	
\S1	Right	9	0	0	23	0	0
E/	Left-Through-Right		1			1	
	-		0			0	
	√ Left	18	0	18	36	0	36
Q	↓ Left	10	1	10	30	1	30
Ž	← Through	131	0	149	95	0	131
ESTBOUND	Through-Right		0	110		0	101
ST	Right	489	1	417	258	1	105
WE	Left-Through-Right		0			0	
	├─ Left-Right		0			0	
		٨	lorth-South:		٨	lorth-South:	
	CRITICAL VOLUMES		East-West:	443		East-West:	246
			SUM:	812		SUM:	677
	VOLUME/CAPACITY (V/C) RATIO:			0.541			0.451
V /	C LESS ATSAC/ATCS ADJUSTMENT:			0.441			0.351
	LEVEL OF SERVICE (LOS):			A			Α
	LEVEL OF SERVICE (LOS).			A			A





I/S #:

PROJECT TITLE:

North-South Street: Main Street East-West Street: Horizon Ave

Scenario: CUMULATIVE + PROJECT CONDITIONS

Count Date: Analyst: Date:

AM PM							
	No. of Phases			2			2
	Opposed Ø'ing: N/S-1, E/W-2 or Both-3?			0			0
	Right Turns: FREE-1, NRTOR-2 or OLA-3?	NB 0	SB	0	NB 0	SB	0
	ATSAC-1 or ATSAC+ATCS-2?	EB 0	WB	0	EB 0	WB	0
	Override Capacity			2			2
	Override dapacity		No. of	Lane		No. of	Lane
	MOVEMENT	Volume	Lanes	Volume	Volume	Lanes	Volume
	↑ Left	19	1	19	12	1	12
	← Left-Through		0			0	
0	↑ Through	386	1	193	208	1	104
	├→ Through-Right		1			1	
R	['] Right	0	0	0	0	0	0
NORTHBOUND	Left-Through-Right		0			0	
	Left-Right		0			0	
	5 - 1 att						
9	↓ Left Left-Through	0	0 1	0	0	0	0
Į	↓ Through	171	0	94	548	0	279
l B	→ Through-Right	171	1	J- 1	340	1	219
SOUTHBOUND	Right	16	0	94	9	0	279
0	← Left-Through-Right		0			0	
S	→ Left-Right		0			0	
					1		
	Left	0	0	0	0	0	0
Į	→ Left-Through	•	0	•		0	•
ğ	→ Through → Through-Right	0	0 0	0	0	0 0	0
l ji	Right	0	0	0	0	0	0
EASTBOUND	→ Left-Through-Right	U	0	Ū		0	U
ш	✓ Left-Right		0			Ö	
					·		
	√ Left	8	0	8	6	0	6
ESTBOUND			0			0	
١٥	← Through ← Through-Pight	3	0	33	3	0	19
∥ E	i i ii ougii-nigiit	00	0	•	10	0	^
EST	Right	22	0 1	0	10	0 1	0
₹	Left-Through-Right Left-Right		0			0	
	,	٨	lorth-South:	193	٨	lorth-South:	291
	CRITICAL VOLUMES	"	East-West:	33		East-West:	19
			SUM:	226		SUM:	310
	VOLUME/CAPACITY (V/C) RATIO:			0.151			0.207
V.	/C LESS ATSAC/ATCS ADJUSTMENT:			0.131 0.075			0.207 0.107
<u> </u>	LEVEL OF SERVICE (LOS):			Α			Α





I/S #: 2 PROJECT TITLE:

North-South Street: Main Street East-West Street: Abbot Kinney

Scenario: CUMULATIVE + PROJECT CONDITIONS

Count Date: Analyst: Date:

AM PM							
	No. of Phases			2			2
	Opposed Ø'ing: N/S-1, E/W-2 or Both-3?			0			0
	Right Turns: FREE-1, NRTOR-2 or OLA-3?	NB 0	SB	0	NB 0	SB	0
	ATSAC-1 or ATSAC+ATCS-2?	EB 0	WB	0	EB 0	WB	0
	Override Capacity			2			2
	Override dapacity		No. of	Lane		No. of	Lane
	MOVEMENT	Volume	Lanes	Volume	Volume	Lanes	Volume
	↑ Left	9	1	9	10	1	10
	← Left-Through		0			0	
O	↑ Through	396	1	226	221	1	126
9	→ Through-Right		1			1	
R	['] Right	55	0	55	31	0	31
NORTHBOUND	Left-Through-Right		0			0	
	Left-Right		0			0	
	5 21,5#	444	4		000	4	200
9	↓ Left ↓ Left-Through	144	1 0	144	306	1 0	306
Ž	↓ Through	137	1	74	501	1	255
B	→ Through-Right	107	1	, 4	301	1	200
SOUTHBOUND	Right	10	0	10	8	0	8
0	← Left-Through-Right		0			0	
S	→ Left-Right		0			0	
					,		
		26	0	26	20	0	20
Ĭ	→ Left-Through	400	0	444	4.07	0	040
EASTBOUND	→ Through → Through-Right	109	0 0	144	167	0 0	213
ı i	Right	9	0	0	26	0	0
ΑŞ	→ Left-Through-Right	J	1	Ū	20	1	U
ш	∠ Left-Right		0			0	
	√ Left	18	0	18	36	0	36
ESTBOUND			1			1	
g	← Through	131	0	149	95	0	131
TB	Through-Right	400	0	44-	050	0	405
ĘS	Right	489	1	417	258	1	105
>	Left-Through-Right Left-Right		0 0			0 0	
	, —·············	٨	lorth-South:	370	٨	lorth-South:	432
	CRITICAL VOLUMES		East-West:	443		East-West:	249
			SUM:	813		SUM:	681
	VOLUME/CAPACITY (V/C) RATIO:			0.542			0.454
V.	C LESS ATSAC/ATCS ADJUSTMENT:			0.442			0.354
	LEVEL OF SERVICE (LOS):						
<u> </u>	LEVEL OF SERVICE (LOS):			Α			Α